DIVISION OF CONSTRUCTION AND RESEARCH TRANSPORTATION LABORATORY

RESEARCH REPORT

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CALIFORNIA CITY AND COUNTY SKID RESISTANCE PROGRAM

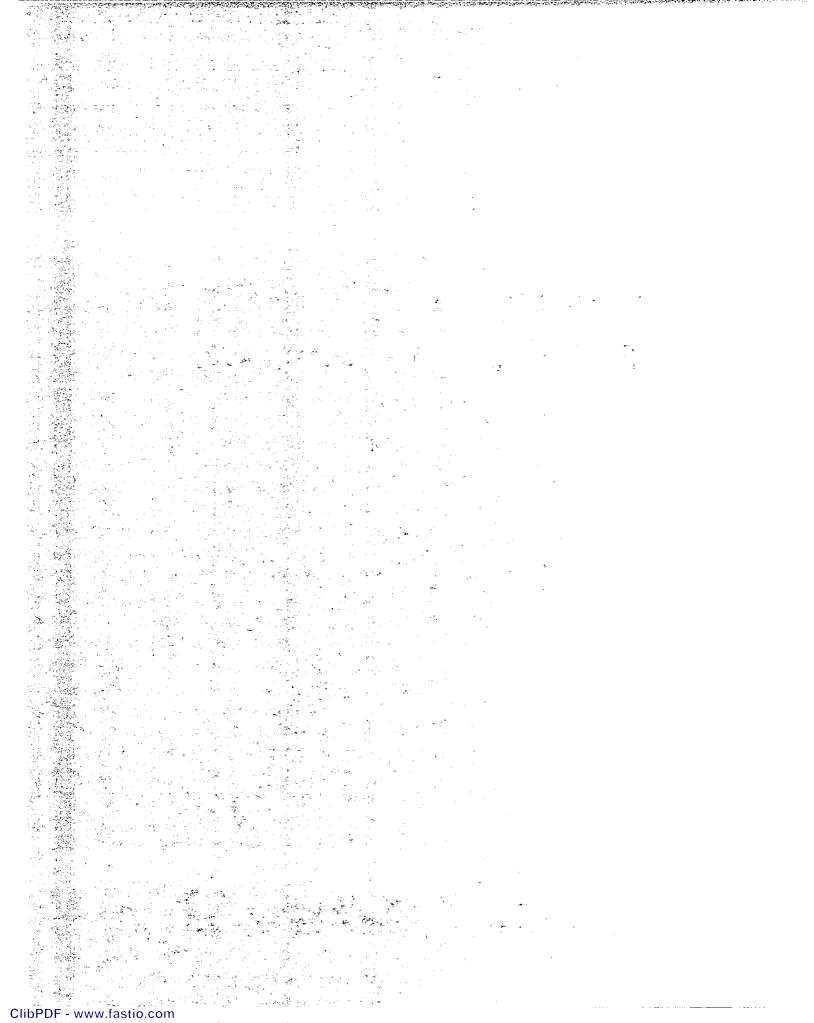
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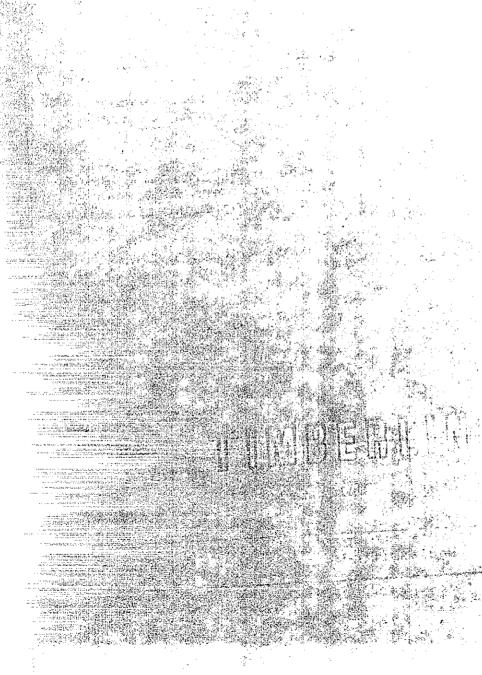
CA-DOT-TL-3666-1-75-36

August 1975





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STATE OF CALIFORNIA DEPARTMENT OF TRANSPORTATION DIVISION OF CONSTRUCTION AND RESEARCH TRANSPORTATION LABORATORY

Mr. R. J. Datel Chief Engineer

Dear Sir:

I have approved and now submit for your information this final project report titled:

CALIFORNIA CITY AND COUNTY SKID RESISTANCE PROGRAM

Study made by	P	avement Branch
Under the Supervision of .	J	ohn B. Skog, P. E.
Principal Investigator .	J	ames A. Matthews, P. E.
Co-Investigator		obby G. Page, P. E.
Report Prepared by		Bobby G. Page, P. E.

Very truly yours,

GEORGE A HILL

Chief, Office of Transportation Laboratory

Attachment

ACKNOWLEDGMENT

The authors wish to express their appreciation to Henry McCormack, assisted by Sylvester Dalske and Harvey Sterner, for their efforts in summarizing the test data for the individual agency reports. Also appreciated are the clerical efforts by Eileen Avender, Lydia Burgin, Kay Thompson and Tina Clubb, and the data computerization efforts by Kollette Kidd and Ralph Weber.

Lloyd Batham III and Gene Stucky performed the skid testing for the local agencies and Martin Bianco scheduled the testing.

This project is a part of the California Traffic Safety Program and was made possible through the support of the Office of Traffic Safety, State of California, and the National Highway Traffic Safety Administration.

The opinions and conclusions expressed in this publication are those of the Project Director, Coordinator and Supervisor, and not necessarily those of the State of California or the National Highway Traffic Safety Administration.

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INTRODUCTION

The skid resistance of a pavement surface is one important factor that influences highway safety in wet weather. In many cases the occurrence of wet pavement accidents has been necessary before the responsible agency recognized that a skidding problem may exist. A program to locate potential problem areas before accidents occur is very desirable and, therefore, this project was initiated.

The California Department of Transportation Laboratory performed this skid resistance inventory program under the general guidelines and financial support of the National Highway Traffic Safety Administration through the California Office of Traffic Safety.

The Transportation Laboratory has several years' experience in skid resistance research. This previous work, performed with the California portable skid tester, has resulted in development of minimum skid resistance requirements for newly constructed portland cement concrete pavements and in tentative minimum requirements for remedial action on existing state highways (1). Various degrees of polishing occur on the pavement surfaces and some friction factors drop significantly under heavy traffic loads, so an inventory program is desirable to maintain current information on roadway surface values. The Transportation Laboratory completed a skid resistance inventory of the California state highway system in March, 1973 (2).

A comprehensive report that evaluates skid resistance requirements and recommends minimum interim skid numbers is National Cooperative Highway Research Program Report 37 (3). Recommended values in this report compare favorably with California's experience (1) and, therefore, they are suggested as guidelines for surface evaluation. Utilizing these guidelines, it is expected that areas with a potential

for a high incidence of wet pavement accidents can be located. Also, these guidelines should be helpful in evaluating the more common types of surface treatment, or construction procedure, that could be used to provide a satisfactory level of skid resistance for a minimum expenditure. It is highly probable that the values referred to in these guidelines can be refined to reflect particular variations in city or county requirements by comparing wet pavement accidents to skid number. The ratio of wet pavement accidents to total accidents, roadway geometrics, advisory and regulatory traffic speeds, ADT and age of the pavement surfacing may be important considerations when making a comparison.

The objective of this project was to measure the skid resistance values of the various pavement surfaces for the California cities or counties that requested this service.

The skid numbers of the pavement surfaces were obtained with towed trailer skid testers because they perform at or near existing traffic speeds and do not require lane closure. These test units comply with ASTM E274 Standards. Skid numbers are defined in ASTM E274 as the tractive force between the tire and the wetted pavement multiplied by 100 and divided by the dynamic vertical load on the test wheel. They are obtained with a locked wheel, ribbed tire (ASTM E249) on a 0.02 inch water layer. The tractive force is obtained at a constant velocity and is averaged for a one-second interval. Subscripts are used with the symbol SN to denote the test speed. All skid numbers were measured at 40 MPH or converted to equivalent 40 MPH values and are thus designated SN₄₀.

Each city or county that requested skid test measurements provided the Laboratory with a list of roadways to be tested. Although emphasis was placed on the selection of roadways with a posted speed of 40 MPH or greater, some roadways with a lower speed limit were tested. In this way, information was provided for a greater variety of surface treatments. The local agencies were encouraged to select recently placed surfaces to compare with those subjected to considerable traffic as well as areas where skid resistance problems might occur. Test data for each agency was summarized and reported to them along with tentative guidelines for minimal skid numbers so that the surfaces could be evaluated. Approximately 9200 lane miles of roadway have been represented under this project.

SUMMARY

Towed trailer skid tests were performed for California City and County agencies that requested the service. Participation in this program varied considerably, based upon each agency's needs. Test result data representing commonly used surfaces are summarized in this report. Results for each city or county have been transmitted to the individual agency in an earlier report. In general, a rather high level of skid resistance values were measured for the popular or common surface types encountered. Very few of the measured surfaces had minimal values because of aggregate polishing. Each agency is expected to relate the skid numbers to wet pavement accidents for various classes of vehicle maneuvering and either substantiate the values that have been submitted as guidelines or establish minimal numbers that relate better to city and county roadways.

FINDINGS

In general, the measured skid numbers were higher than the tentative minimum standards as proposed by NCHRP Report 37 (3).

Test values of skid numbers that might be considered minimal or low on asphalt treated or asphalt concrete surfaces appear to be due to an excess of asphalt at the surface.

Portland cement concrete surfaces did reveal some low skid number values believed to be caused by polishing in the outside or truck lanes and at stop approaches where braking and accelerating actions take place.

A statistical listing of the SN_{40} values for the more common surface types follows:

Surface Type	Number of Tests	Overall Mean Value	Range of Individual Mean Values	Range of Individual Std. Dev.
Dense Graded Asphalt Concrete	14,062	43.8	55.7 - 32.9	11.7 - 5.4
Chip Seal	4,080	50.0	66.2 - 39:0	12.0 - 4.3
Portland Cement Concrete	1,200	38.5	64.0 - 35.3	9.3 - 2.7
Slurry Seal	905	45.7	57.3 - 38.1	18.0 - 4.1
Open Graded Asphalt Concrete	148	46.2	47.9 - 41.5	6.2 - 2.2
Road Mix Asphalt Surfacing	143	54.9	57.8 - 47.2	10.0 - 3.8
Fog Seal	121	37.1	42.0 - 36.3	15.9 - 6.1
Oiled Gravel	92	62.4		11.6
Sand Seal	11	57.0		0°13

Other surface treatments were tested as requested by some of the California cities and counties participating in this program. However, a sufficient amount of testing was not performed that would permit an analysis leading to any specific conclusions relative to the skid resistant qualities of these treatments. Test result data on the limited amount of testing accomplished are on file and available for review at the Transportation Laboratory, 5900 Folsom Boulevard, Sacramento.

DISCUSSION

The skid test data obtained for local agencies may be useful in two ways. First, areas having relatively low skid numbers may be identified before the occurrence of wet pavement accidents. Each city and county may apply the data toward this goal. Second, commonly used surface types may be compared for their skid resistance characteristics including the polish resistance of aggregate and construction procedures. The extent of comparison data varied considerably for the local agencies depending upon the availability of surface types and the extent that an agency desired to participate. A list of the local agencies and their involvement is shown in Appendix 1.

The test data tabulated in the Findings section of this report shows that the popular or common pavement surface types can be constructed to a satisfactory level of skid resistance with available materials. A rather large variation in the extremes of the individual standard deviation and mean test values for each surface type reflect the many variables that exist in constructing and maintaining a uniform skid resistant surface. Conclusions should not be drawn from this summary regarding a surface type preference or rejection because the surface type test locations were selected for several reasons. Each agency was encouraged to include roadways that might be potential problem surfaces, surface types that are generally used by the agency, and surface types that were recently placed and the same types subjected to considerable traffic so that an evaluation of performance could be made by the agency. Therefore, the lack of uniform representation of pavement surfaces by some agencies may have biased the values in this summary to an extent that they could be misleading. Also, the length of test sections, their age and exposure to traffic and the conditions

relating to the construction of the surfacing are important considerations for an evaluation. To compare a surface type that has been exposed to light traffic to one subjected to heavy traffic or to one that experienced some unusual construction problems might result in the rejection of a superior surface type that could be placed at less cost than the type accepted. Therefore, each agency should consider all pertinent factors when evaluating the measured test data that has been previously transmitted on data summary sheets like those included in the sample report in Appendix 2. This format is such that average skid numbers are presented for each test section of each roadway. Also, individual test values and the respective locations are listed for any areas of interest within this section. Specifically identified were the test locations of intersections, patched areas, bridge decks, and bleeding asphalt surface areas. In addition to surface type comparisons, this test data can be used in a comparison with wet pavement accidents to substantiate the tentative minimal skid numbers that were proposed in our report, or to establish minimal numbers that relate better to city and county roadways.

In relating skid numbers to wet pavement accidents, several parameters are worth considering. A ratio of the wet pavement accidents to total accidents should provide an indication as to the importance of the pavement surface in an accident area. A relatively low ratio would imply that factors other than skid resistance should be investigated. A second consideration is that of geometric design. It is probable that tight curves, weaving and merging sections and intersection areas were braking is common will require a higher skid number than tangent sections or sweeping curves might require. Other factors that might be considered are the traffic volume, percent of time the pavement is wet, and the consistency in total design.

A recommendation was made in each individual report that the findings of a skid number to wet pavement accident comparison be transmitted to the Transportation Laboratory. This information will be assessed for the purpose of establishing minimal numbers for city and county roadways in California.

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- 1. Zube, E., Skog, J. B., Kemp, G. R., and Stucky, G. J.,
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 Pavement Surfaces," State of California, Department of
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 Department, Research Report 633126-2, February, 1968.
- 2. Sherman, George B., Matthews, James A., Page, Bobby G.,
 "Statewide Skid Resistance Inventory," State of California
 Department of Public Works, Division of Highways, Materials
 and Research Department, Report 673500, March 1973.
- 3. Kummer, H. W., and Meyer, W. E., "Tentative Skid Resistance Requirements for Main Rural Highways," NCHRP Report 37, 1967, Highway Research Board, Washington, D. C.

APPENDIX 1

City-County Participation

City or County

Surface Type

		Chip Seal	(STULK Seal	OGAC		roy sear Oiled Gravel	Sand Seal	Rubberized Chip Seal	Reclamite	Heater Remix	Expanded Shale	Ž.		Gilsabind	Oiled Dirt	Petroset	Pliopave	Epoxy PCC	FD	Coal Tar Slurry		Chevron Jet Seal	Macadam
Alameda Co.	×	×	k :	×	`														×					
Anaheim	×			×						×				×	<u>.</u>				n	v	v	×	v	
Contra Costa Co.		×	×:							×			×		×					"		_	•	
El Dorado Co.	×									n			^		^									
Fresno Co.		×		•																				
Garden Grove	×			X																				
Imperial Co.	×	×:	×																					
Kings Co.	. *	×	>	*	×	:										×							,	×
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Marin Co. Monterey Co.	×		×	X.																				
Nevada Co.	*																							
Orange Co.	- X .		×																					
Plumas Co.	×	;	×Þ	4							•			×	×		×	×						
Riverside	× :	.		< :	ų.	*	:										•							
Sacramento Co.			\																					
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San Francisco	×																							
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San Mateo Co.	×		×		×																			
Santa Barbara Co.	×			×	• •	×	(. :	×												
Santa Clara Co.		× :																						
Solona Co.	* *	4 >	< >	4																				
Sonoma Co.	×			;	X																			
Sutter Co.	×				×		×																	
Tulare Co.	×				×			×		×	×													
Ventura Co.	×	× :	× >	×																				
Walnut Creek	×																							

APPENDIX 2

SAMPLE REPORT

State of California
Department of Transportation
Division of Construction and Research
Transportation Laboratory

SKID RESISTANCE VALUES OF PAVED ROADS IN EL DORADO COUNTY

Project Director John B. Skog, Supervising Materials and Research Engineer, Pavement

Project Supervisor James A. Matthews, Senior Materials

and Research Engineer

Project Coordinator Bobby G. Page, Associate Materials

and Research Engineer

Report by B. G. Page

H. D. McCormack

G. S. Stucky

INTRODUCTION

The California Department of Transportation Laboratory performed this skid resistance inventory program under the general guidelines and financial support of the National Highway Traffic Safety Administration through the California Office of Traffic Safety.

The Transportation Laboratory has several years experience in skid resistance research. This previous work, performed with the California portable skid tester, has resulted in development of minimum skid resistance requirements for newly constructed portland cement concrete pavements and in tentative minimum requirements for remedial action on existing state highways(1). Various degrees of polishing occur on the pavement surfaces and some friction factors drop significantly under heavy traffic so an inventory program is desirable to maintain current information on roadway surface values. A skid resistance inventory of the California state highway system was completed by the Laboratory in March, 1973(2).

Excluding structural overlays and overlays to improve the riding qualities, the primary justification for corrective treatment of pavement surfaces has been the occurrence of accidents, especially wet weather accidents. A program to locate potential problem areas before accidents occur is very desirable and, therefore, this project was established.

Towed trailer skid testers are used because they perform at or near existing traffic speeds and do not require lane closure. These test units comply with ASTM E274 Standards. Values measured with these units are termed skid numbers and are defined in ASTM E274 as the tractive force between the tire and the wetted pavement multiplied by 100 and divided by the dynamic vertical load on the test wheel. They are obtained with a locked wheel, ribbed tire (ASTM E249) on a 0.02 inch water layer. The tractive force is obtained at a constant velocity and is averaged for a one-second interval. Subscripts are used with the symbol SN to denote the test speed. All skid numbers in this report were measured at 40 MPH or converted to equivalent 40 mph values and are designated SN_{40} .

A comprehensive report that evaluates skid resistance requirements and recommends minimum interim skid numbers is National Cooperative Highway Research Program Report 37(3). Recommended values in this report compare favorably with California's experience(1) and, therefore, they are suggested as guidelines for surface evaluation.

The objective of this project was to measure the skid resistance values of the various pavement surfaces within the County.

Utilizing the established criteria, it is expected that areas with a potential for a high incidence of wet pavement accidents can be located. Surface treatments that have been subjected to considerable traffic, and recently-placed new surfaces and surface treatments were specifically tested to aid your agency in optimizing resurfacing recommendations.

CONCLUSIONS

For most of the pavement surfaces tested, the skid numbers exceed the tentative minimum skid number requirements for rural highways as defined by National Cooperative Highway Research Program Report 37.

Some of the pavement surfaces appear to have marginal skid resistance values. Areas having a high ratio of asphalt to aggregate at the surface, referred to as bleeding or rich (impending bleeding), exhibit lower skid numbers.

RECOMMENDATIONS

It is recommended that a comparison of wet weather accidents to skid numbers be made for the purpose of comparing the proposed minimal skid numbers to local data. The ratio of wet weather accidents to total accidents, geometric considerations, ADT and age of the pavement surfacing may be of particular importance for this comparison. We request that these findings be transmitted to the Transportation Laboratory for a compilation of city-county minimum skid number for intersections, curves and urban locations.

PROCEDURE

As requested by Mr. G. Arthur Court, J lirector of Public Works, tests were performed on selected pavement surfaces within El Dorado County.

Skid test values and their respective locations are listed in the attached data sheets.

The numeric average and the range of skid number values are listed for each test section. In addition, the location and the skid number is listed for special individual tests. Generally, special tests were made for intersections and areas having a surface type different from the usual such as a patch or a bleeding asphalt surface. The odometer mileage listed is referenced from the first street noted in the test limits. Tests were performed in the right wheel track at 40 mph unless situations such as a curve required a lower test speed. Using previously standardized conversion charts, all low speed test values have been converted to equivalent 40 mph skid numbers.

DISCUSSION

The interpretation of the measured skid data can best be made by comparing test values with proposed minimal skid numbers for the same test speed. National Cooperative Highway Research Program Report 37, "Tentative Skid-Resistance Requirements for Main Rural Highways(3) lists minimal numbers as follows:

Mean Traffic Speed			SN ₄₀
20			
30	• '		31
40			33
50			37
60			41

These recommended skid numbers should meet the frictional needs of normal vehicle maneuvers, assuming that the vehicle is in good mechanical condition and that the tires have a tread depth of at least 1/10 inch. 'badway geometrics may influence the minimal numbers somewhat. Higher skid numbers may be required in areas where sudden vehicle maneuvers are probable. Therefore, a visual inspection to determine the likelihood of normal or severe vehicle movements is important in the implementation of a corrective treatment program. Locations having a comparatively high ratio of wet weather accidents to total accidents can be used to assist your agency in determining critical or minimal skid numbers, providing the roadway geometrics are considered.

The measured SN_{40} values for each surface type tested in El Dorado County are as follows:

Surface Type	Number of <u>Tests</u>	Mean SN ₄₀	Standard Deviation
Chip Seal	239	43.6	14.0
DGAC	194	45.7	11.7
Slurry Seal	26	57.3	7.3
PCC	2	56.0	

Based on the proposed minimal skid numbers, most of the pavement surfaces exceed the minimum requirements for main rural highways. A few areas have a high ratio of asphalt to aggregate at the pavement surface. This condition is referred to as bleeding and generally results in low skid numbers. A chip seal is a poor corrective treatment for a severe or moderate bleeding condition because the new chips soon become imbedded, resulting

in little or no skid resistance improvement. Successful correction has been realized by heating and scraping the excess asphalt prior to a thin asphalt concrete overlay.

A 0.04 ft. to 0.06 ft. overlay utilizing 90 percent crushed particles and an aggregate gradation having a porous or open texture is frequently used to correct a bleeding condition without scraping the excess asphalt. This surface is referred to as open graded asphalt concrete in the California Standard Specifications. Skid numbers for an OGAC usually average about 55 for pavements subjected to light vehicular traffic and about 49 for pavements subjected to considerable truck traffic.

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- 1. Zube, E., Skog, J. B., Kemp, G. R., and Stucky, G. S., "Field and Laboratory Studies on Skid Resistance of Pavement Surfaces", State of California, Department of Public Works, Division of Highways, Materials and Research Department, Research Report 633126-2, February, 1968.
- 2. Sherman, G. B., Matthews, J. A., Page, B. G., "Statewide Skid Resistance Inventory", State of California, L.P.W. Division of Highways, Materials and Research Department, March 1973.
- 3. Kummer, H. W. and Meyer, W. E., "Tentative Skid Resistance Requirements for Main Rural Highways", NCHRP Report 37, 1967, Highway Research Board, Washington, D.C.

STATE OF CALIFORNIA DEPARTMENT OF TRANSPORTATION DIVISION OF HIGHWAYS

County Fl Dorado		DIV. TRANSI	DIVISION OF HIGHWAYS RANSPORTATION LABORATORY	F HIGHWOOD LABO	AYS RATORY		Test]	Method	ASTM E-274
					ASTM	Skid	Numbers	1 E	
Locations and Limits of Test	Test Lane	Wheel Track	Surface Type	No.of Tests	Range SN40	Avg SN40	Unusual Cond.	SN Odom 4	N 40 Comments
Cameron Park Drive from U.S50 to Green Valley Road		Rt "	DGAC Slurry	10	58-66	64	* H	0.00 -	© U.S50 Bet. 0.4 & 3.18 miles
	=		Seal				I	3.27 -	Green Valley Road
Cedar Ravine Road from Pleasant Valley Road to Darlington	N/B	" tt	CSAC	15	43-68 35-45	56	н н	0.00 -	Pleasant Valley Rd. Bet 0.1 & 4.25 miles Bet. 4.4 & 5.1 miles Darlington
Cold Springs Road from Placerville City Limits to State Rte 49	N/B " "	Rt "	DGAC Slurry	9	39-57 46-66	46 I	I Fog Seal		Placerville City Limit Bet. 0.07 & 2.0 miles Bet. 2.3 & 3.8 miles
	= = =	2 2 2	Sear CSAC DGAC	ra m a	52-61 41-50 14-64	57 455	Overlay		Bet. 4.1 & 4.4 miles Bet. 4.7 & 5.3 miles Bet. 4.6 & 7.21 miles
		= = =)) ⊢ ⊢ ⊢	r S)	* * BB1 T BB1	6.48 18 6.74 22 7.04 14 7.34 -	State Rte 49
Jct. State Rte 49 to 0.83 miles south	S/B	Rt	CSAC	9	30-44	3.4	Н	0.00	State Rte 49 Bet. 0.29 & 0.83 miles
<pre>I* Intersection Bl** Bleeding</pre>								·	
DH-TL-3110 (New 4/74)									

STATE OF CALIFORNIA
DEPARTMENT OF TRANSPORTATION

		NAGT	VIDION OF MICHIGATORY	N LABORAT	RATORY		Test	Method	ASTM	M E-274
unty Yang	Test		Surface	No.of	Range SNA	Skid Skid Sna		mo po	SN 40	Comments
Locations and Limits of Test La	Lane	Track	ad XI	16363	7	†	-0110			1
El Dorado Hills Blvd. from N/B U.S50 to Francisco Drive "		Rt.	CSAC	9 -1	48-56	53	H	0.00	- 84 8 ж	<u>g</u> u.s50 Bet. 0.1 & 1.6 miles
	<u> </u>	E	Seal CSAC	'n	43-58	53	н	3,65	1	Bet 2.2 & 3.5 miles Francisco Drive
Francisco Drive from El N/B Dorado Hills Blvd. to Green " Valley Road.		Rt	CSAC	9	33-55	45	н н	0.00	1 1	El Dorado Hills Blvd Bet. 0.04 & 0.49 mil Green Valley Road
Green Valley Rd. From Francisco Drive to Placerville City Limits		## = = = = = = = = = = = = = = = = = =	DGAC CSAC " " " " " " " " " " " " " " " " " " "	44' 0	16-66	ひ 4. ひ 心	Rich Rich Rich Rich Rich Bl Bl	0.00 0.00 0.00 1.0	1 21232112 1 299828697	Francisco Drive Bet. 0.1 & 2.8 miles Bet. 3.1 & 15.9 miles

STATE OF CALIFORNIA DEPARTMENT OF TRANSPORTATION DIVISION OF HIGHWAYS

ASTM E-274		N 40 Comments	- Debbie Lane 27	Bet. 1.52 & 4.04 miles	Bet. 4.78 & 4.87 miles	Sac/E.D. Col	- K U.S50	Bet. 0.5 & 3.5 miles.	28 Ret. 4.67 & 8.4 miles	S.Shingle Spring	- State Rte. 49	Bet. 0.3 & 0	8 & 1.32	1.7 & 1.99	Bet. 2.3 & 3.2 mile Bet. 3.5 & 6.6 mile	Valley Ro	
Method		SN Odom 4			9 9 9	<u> </u>			~ 	<u>.</u> ო	+					<u>8</u>	
	S	<u> </u>	0.00		<u>n</u>	17.4	0.00		4	രസ	00.0	0.		о Т		6.8	,
Test	Numbers	Unusual Cond	≃	ļ.	Rich	Ħ ¤	H	Overlay Patch	B1	Patch I	H	B1	Bl	Rich	Overlay New	; H }	
	Skid	Avg SN40		40	8 6	က က		52	6.0))		46	200	# M	r) u)	
LABORATORY	ASTM	Range SN40		26-52	26-30	† † † † † † † † † † † † † † † † † † †		49-56	٦ ١ ٦			2.1	21-35	0	47-55	1	
<u> </u>	ì	No.of Tests	FI	6	10	7 7		l H H H	ıн ₂	ļ		Η С	ı vo -	⊣ 71	4.0		
TRANSPORTATION		Surface Type	CSAC	; ; = ;	DGAC CSAC	:	CSAC	DGAC "	ב ה מינ	DGAC		DGAC) = =	: =	DGAC "		
TRA		Wheel Track	1	= :	: = :		±) {= = ·	= =	=	·	∓ ₹	= =	: = ,	= =		
		Test	W/B	= :	= =	: =	S/B	= =	E E	E E	S/B	.= =	= =	: =	= =	=	
County El Dorado	STRICK THE P	Locations and Limits of Test	ey Road from to France	Drive.			Latrobe Road From U.S50				Lotus Road from State Rte 49	en Va					

STATE OF CALIFORNIA
DEPARTMENT OF TRANSPORTATION
DIVISION OF HIGHWAYS

County El Dorado		DIV	DIVISION OF F RANSPORTATION	~ ~	IIGHWAYS LABORATORY	-	Test	Test Method ASTM	AS	IM E-274
Lim	Test	Wheel Track	Surface Type	o.of	ASTM Range SN40	Skid Avg SN40		Odom	SN 40	Comments
Missouri Flat Road from State Rte 49 to Green Valley Rd.	N/B	" " " " " " " " " " " " " " " " " " "	DGAC " PCC DGAC	инни	43-48	. 44 . 57 . 8	1 Z	0.00 1.54 1.7 3.46	31 556	State Rte 49 Bet. 0.08 & 1.3 miles Bet. 2.0 & 3.2 miles Green Valley Road
Mt. Aukum Road from Amador Co. Line to Pleasant Valley Rd.	N/B	Rt	CSAC DGAC " CSAC " DGAC	24.0 6.4.2 194.6	33 - 1 - 1 - 6 - 6 - 6 - 6 - 6 - 6 - 6 - 6	0 8 0 8 4 U 0 4 0 0 0 U	I Pavement Change Rich Bridge	0.00	1 29	Ama. Co. Line Bet. 0.03 & 0.14 mile Bet. 0.4 & 1.3 miles Bet. 1.6 & 3.1 miles. Bet. 3.23 & 4.64 mile Bet. 4.9 & 5.93 miles Bet. 6.2 & 12.38 mile Pleasant Valley Road
Newtown Road from Broadway to Pleasant Valley Road	S/B	Rt "	CSAC " "	13 11 4	37 18-64 39-58	224 455	I Bl Bl	0.00	. 1 1	Broadway Bet. 0.4 & 2.55 miles Bet. 2.71 & 4.77 mile Bet. 5.37 & 6.30 mile Pleasant Valley Rd.
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DH-TL-3110 (New 4/74)

County El Dorado		S DEPAI DJ	STATE OF CALIFORNIA DEPARTMENT OF TRANSPORTATI DIVISION OF HIGHWAYS TRANSPORTATION LABORATORY	CALIFORNIA TRANSPORTATION F HIGHWAYS ON LABORATORY	NIA ORTATI AYS RATORY	ON	Test 1	Met <u>h</u> od	ASTM	rM E-274
Limi	Test	Wheel Track	Surface Type	No.of Tests	ASTM Range SN40	Skid SN9 SN40	Numbers Unusual Cond.	Odom	SN 40	Comments
oad sant Valley Rd ay	N/B "	Rt "	CSAC		8-36	22	B1	• •		Pleasant Valley Rd. Bet. 0.68 & 3.75 miles
					47-59 16-34	54 25	B1 I	2.52 6.49) to	Bet. 3.95 & 6.40 miles Bet. 5.93 & 6.05 miles Broadway
North Shingle Road from Green Valley Rd. to U.S50	S/B "	Rt	DGAC	14	41-62	49	H H	0.00	1 1	Green Valley Rd. Bet. 0.10 & 3.94 miles <u>Ø</u> U.S50
Pleasant Valley Road from Mt. Aukum Rd. to State Rte 49	W/B "	Rt "	DGAC Slurry	15 3	43-57 50-58	49	I Overlay	00.00	ſ	Mt. Aukum Road Bet. 0.1 & 5.0 miles Bet. 5.32 & 5.85 miles
	= =	= =	sear DGAC Slurry	0.20	55-61 44-61	വയ				Bet. 6.1 & 6.5 miles Bet. 6.9 & 8.6 miles
	= =	= =	Seal CSAC	м	38-45	42	H	9.51	1	Bet. 9.0 & 9.4 miles State Rte 49
			e de la companya de l							
									en e	

		7	T			
	∠ U.S50 Bet. 0.35 & 0.9 miles Bet. 1.1 & 3.6 miles Green Valley Road	Mt. Ackum Rd. Bet. 0.11 & 11.44 mile U.S50 U.C.	Latrobe Road Bet. 0.06 & 4.8 miles Bet. 5.1 & 7.5 miles Bet. 7.74 & 7.88 miles Mother Lode Drive	Mother Lode Drive Bet. 0.02 & 0.2 miles Bet. 0.35 & 2.55 miles		
1 101	14 45 20 1	1 99	l I	1		
Mobo	0.00 0.10 2.4 3.66	0.00	0.00	0.00		
		нн	I Rich I	I Rich		
Skid Avg SN40	22 51	46	54 26 43	45 25		
ASTM Range SN40	19-24	21-57	40-62 21-29 42-44	42-47	-	
	НФОН	37	16 8 2	3 10	·	
Surface Type	DGAC " CSAC	DGAC	CSAC DGAC	DGAC "		
Wh	# : : :	Rt	Rt	Rt "		
i	N/B 	N/B "	N/B	S/B "	·	
Locations and L	Va	Sly Park Road from Mt. Aukum Rd. to U.S50	South Shingle Road from Latrobe Rd. to Mother Lode Drive	From Mother Lode Drive to 2.55 miles South		
	Locations and Limits of Test Lane Track Type Tests SN40 Cond. Cond. Odom 40	from N/B Range Green Valley N/B Range Track No.of Range Track ASTM Skid Numbers Numbers SN40 Cond. AO Comments from N/B Rt DGAC 1 I 0.00 - Customents Green Valley " Rt DGAC 1 19-24 22 B1 Bet. 0.35 & 0.9 " " " 44-58 51 B1 2.4 20 " " " " Bet. 1.1 & 3.6 m " " " " "	Test Wheel Surface No. of Range Rayg Unusual SN SN40 Cond. Odom 40 Comments	Test Wheel Surface No.of Range R	Ponderosa Road from	Ponderosa Road from